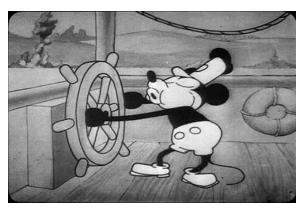
WORCESTER POLYTECHNIC INSTITUTE MECHANICAL ENGINEERING DEPARTMENT

STRESS ANALYSIS ES-2502, D'2020

We will get started soon...



12 May 2020





WORCESTER POLYTECHNIC INSTITUTE MECHANICAL ENGINEERING DEPARTMENT

STRESS ANALYSIS ES-2502, D'2020

We will get started soon...

Lecture 26: Unit 24, 25: Principal Stresses. Mohr's circle

12 May 2020





General information

<u>Instructor</u>: Cosme Furlong HL-152 (774) 239-6971 – Texting Works

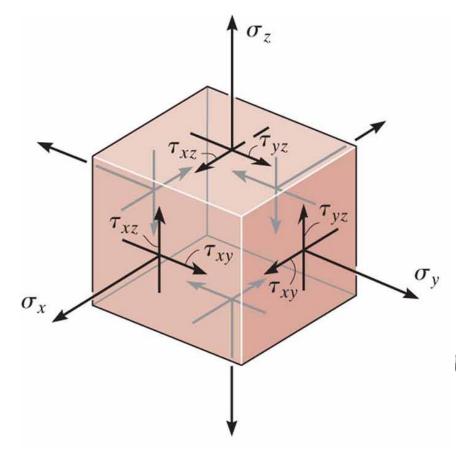
Email: cfurlong @ wpi.edu http://www.wpi.edu/~cfurlong/es2502.html

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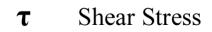


Stress at a point General state of stress. Stress cube in 3D



Notation

σ Normal Stress



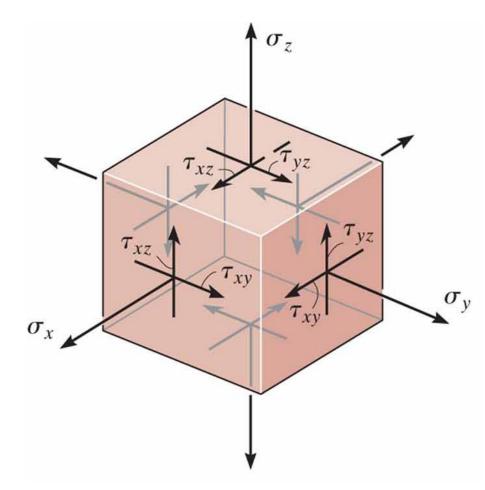
 τ_{xy} Face Direction

Equilibrium conditions require that:

$$\tau_{xy} = \tau_{yx}, \ \tau_{xz} = \tau_{zx}, \ \tau_{yz} = \tau_{zy}$$
Why?



Stress at a point General state of stress. Stress cube in 3D



There are 9 components of stress.

Equilibrium conditions are used to reduce the number of stress components to 6:

$$\tau_{xy} = \tau_{yx}$$

$$\tau_{xz} = \tau_{zx}$$

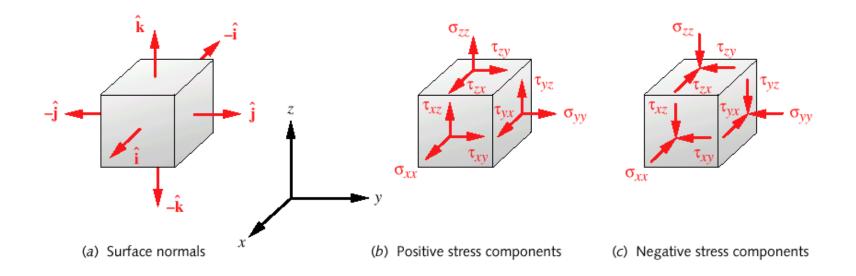
 $\tau_{yz} = \tau_{zy}$



Stress tensor Cauchy stress tensor

$$\begin{bmatrix} \sigma_{xx} & \tau_{xy} & \tau_{xz} \\ \tau_{yx} & \sigma_{yy} & \tau_{yz} \\ \tau_{zx} & \tau_{zy} & \sigma_{zz} \end{bmatrix}$$

Tensors are quantities that are invariant to coordinate transformations (i.e., translations & rotations)

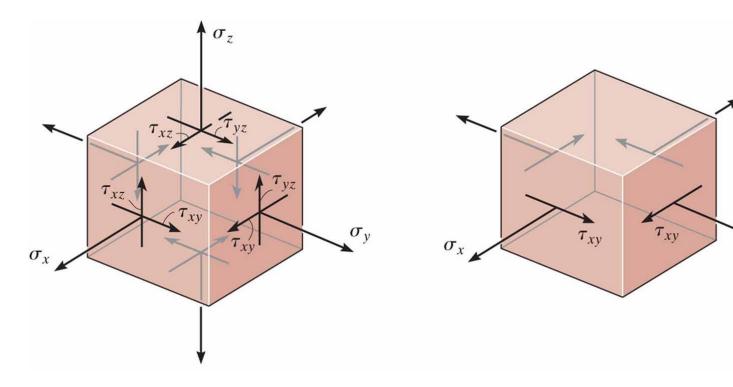




Stress at a point Plane stress

General state of stress

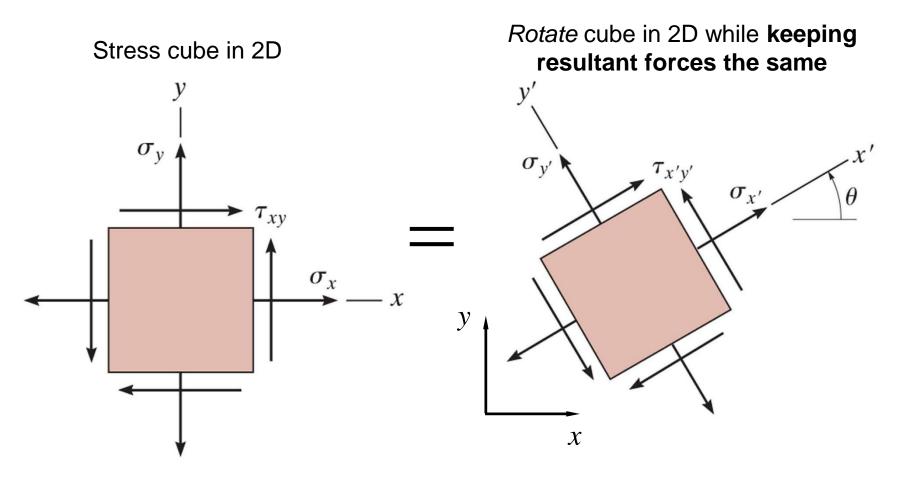
Pane stress



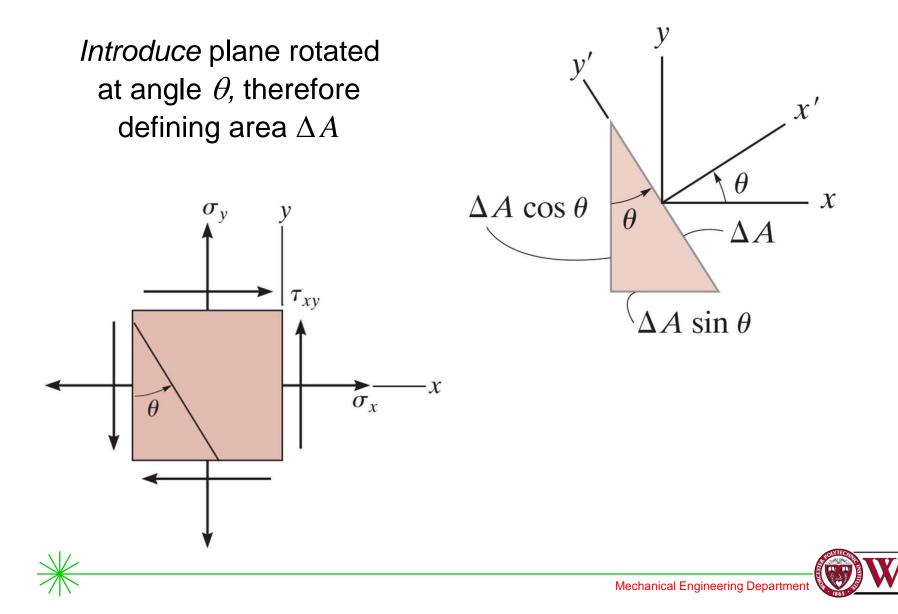


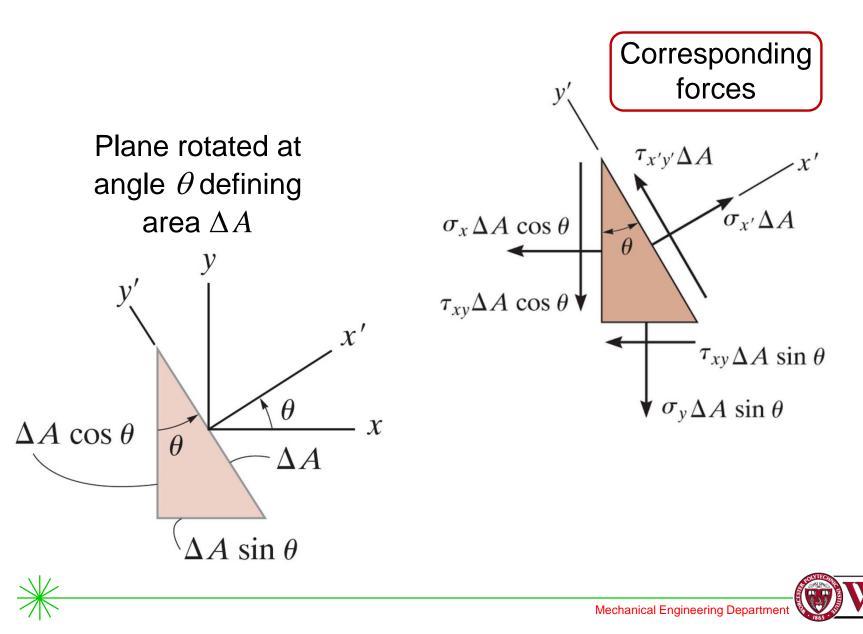


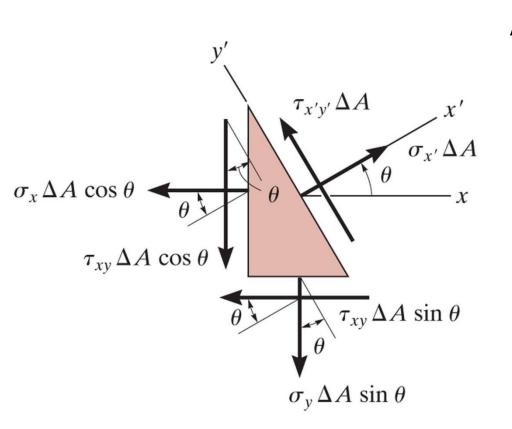
 σ_y









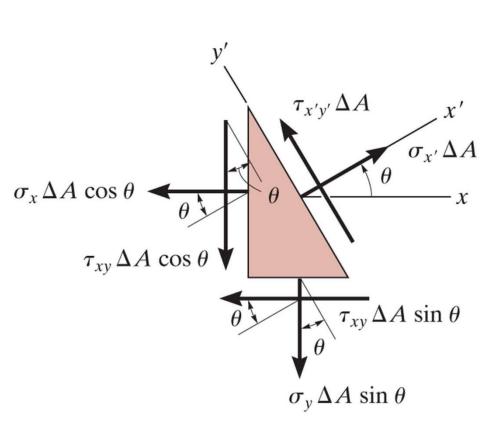


Apply equilibrium conditions:

(1)
$$\sum F_{x'} = 0; + \checkmark$$

(2) $\sum F_{y'} = 0; + 1$





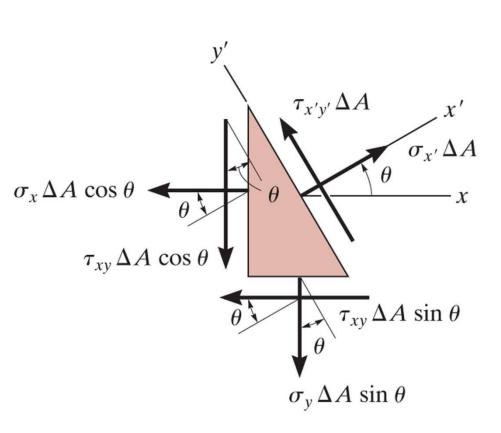
First equilibrium condition:

(1)
$$\sum F_{x'} = 0; + \checkmark$$

- $\sigma_{x'} \Delta A (\tau_{xy} \Delta A \sin \theta) \cos \theta$ $(\sigma_{y} \Delta A \sin \theta) \sin \theta$
 - $-(\tau_{xy}\Delta A\cos\theta)\sin\theta$

$$-(\sigma_x \Delta A \cos \theta) \cos \theta = 0$$



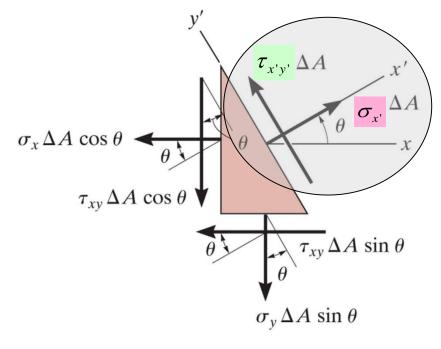


Second equilibrium condition:

(2)
$$\sum F_{y'} = 0; + 1$$

- $\tau_{x'y'} \Delta A + (\tau_{xy} \Delta A \sin \theta) \sin \theta$
 - $-(\sigma_y \Delta A \sin \theta) \cos \theta$
 - $-(\tau_{xy}\Delta A\cos\theta)\cos\theta$
 - $+ (\sigma_x \Delta A \cos \theta) \sin \theta = 0$





Transformation equations become:

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2}\sin 2\theta + \tau_{xy}\cos 2\theta$$

and by setting
$$\theta = \theta + 90^{\circ} \Rightarrow$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$





In-plane principal stresses

In-plane principal <u>normal</u> stresses can be obtained by:

$$\frac{d}{d\theta}\sigma_{x'} = \frac{d}{d\theta} \left(\frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta \right) = 0$$

resulting in an angle of:

$$\tan 2\theta_p = \frac{\tau_{xy}}{(\sigma_x - \sigma_y)/2}$$

which is substituted into transformation equation to lead to the **principal stresses (Max/Min)**:

$$\sigma_{1,2}_{(Max,Min)} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$



Maximum and Minimum in-plane shear stress

Max/Min in-plane shear stresses can be obtained by:

$$\frac{d}{d\theta}\tau_{x'y'} = \frac{d}{d\theta} \left(-\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \right) = 0$$

resulting in an angle of: $\tan 2\theta_s = \frac{-(\sigma_x - \sigma_y)/2}{\tau_{xy}}$

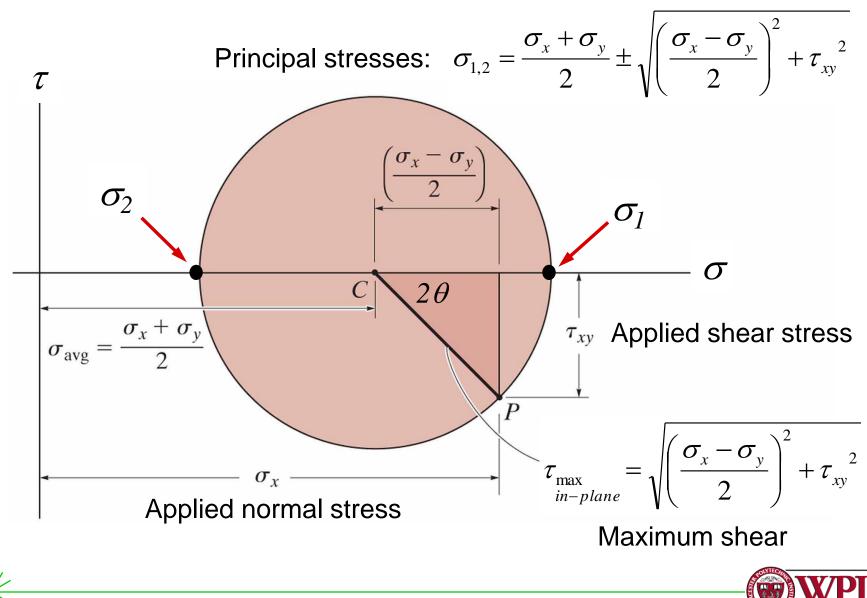
which is substituted into transformation equation to lead to the **maximum in-plane shear stress (Max/Min)**:

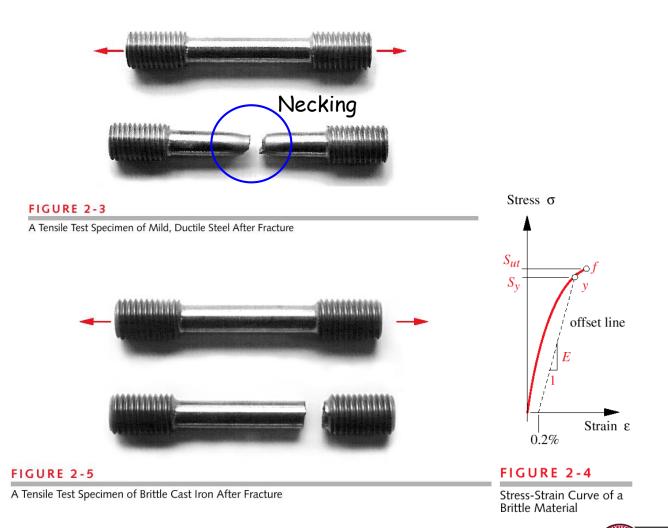
$$\tau_{\max/Min}_{in-plane} = \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$





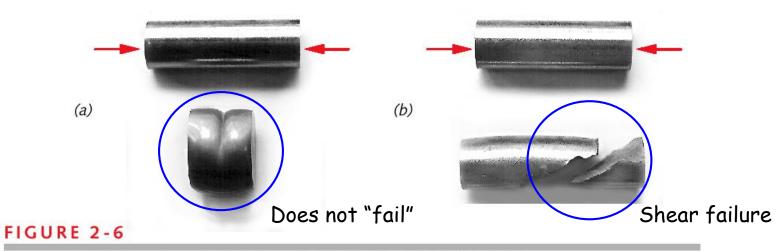
Mohr's circle (developed by Otto Mohr in 1882)









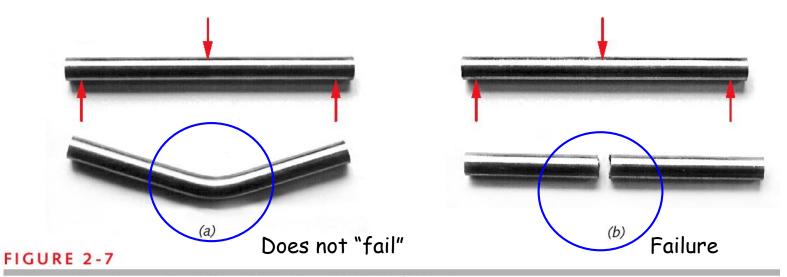


Compression Test Specimens After Failure (a) Ductile Steel (b) Brittle Cast Iron

Even materials: same behavior in tension and in compression.







Bending Test Specimens After Failure (a) Ductile Steel (b) Brittle Cast Iron





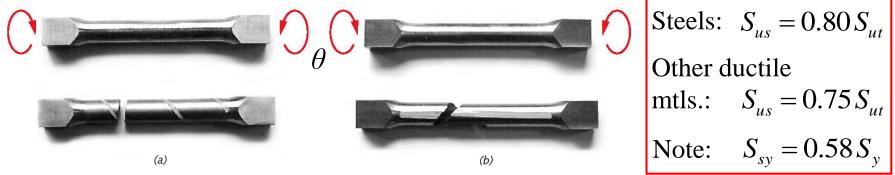


FIGURE 2-8

Torsion Test Specimens After Failure (a) Ductile Steel (b) Brittle Cast Iron

Stress-strain relation
(torsion):
$$\tau = \frac{Gr\theta}{l_o}$$

Modulus of rigidity:

$$G = \frac{E}{2(1+\nu)}$$

Table 2-1	
Poisson's Ratio v	
Material	ν
Aluminum	0.34
Copper	0.35
Iron	0.28
Steel	0.28
Magnesium	0.33
Titanium	0.34

Ultimate shear strength (torsion): $S_{us} = \frac{T_{(break)}r}{J}$

Not uniform stress distribution; (in some cases, thin-walled tubes are preferred for this test, why?)



Reading assignment

- Chapter 9 of textbook
- Review notes and text: ES2001, ES2501





Homework assignment

• As indicated on webpage of our course



