

WORCESTER POLYTECHNIC INSTITUTE MECHANICAL ENGINEERING DEPARTMENT

DESIGN OF MACHINE ELEMENTS ME-3320, B'2025

Lecture 08-09

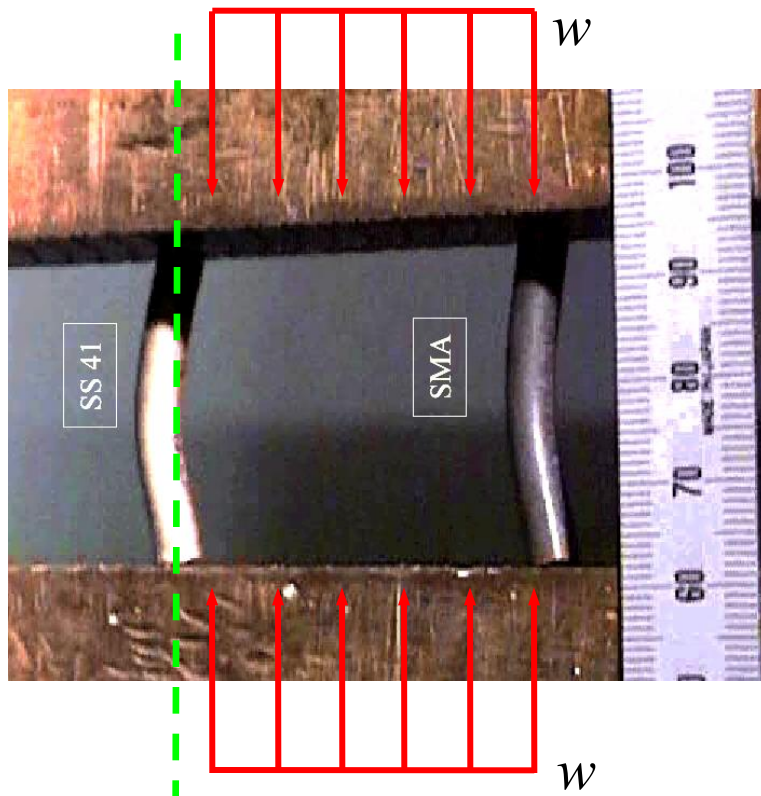
November 2025

Optional



Axial compression -- columns

Examples



Concentric
load



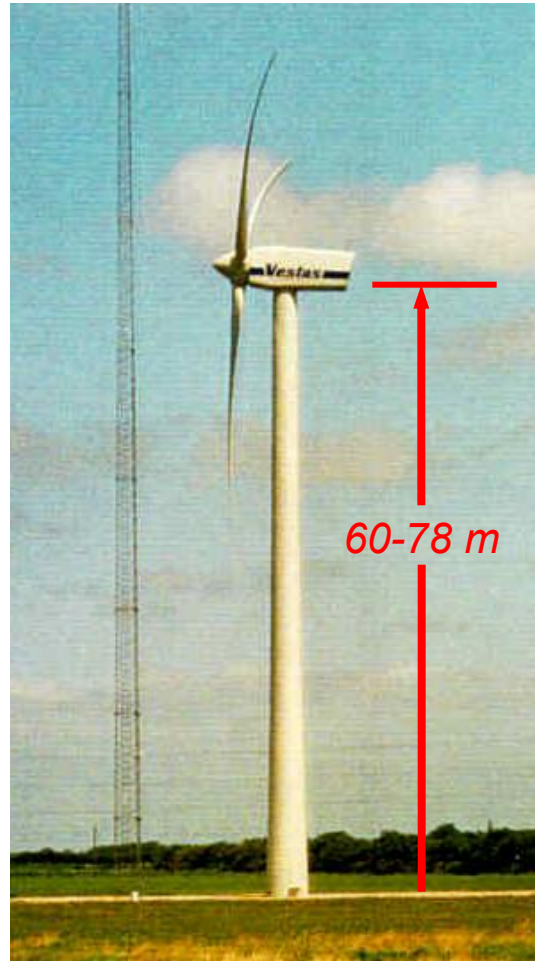
Eccentric
load



Axial compression -- columns

Examples

Vestas' V80-2.0 MWatt



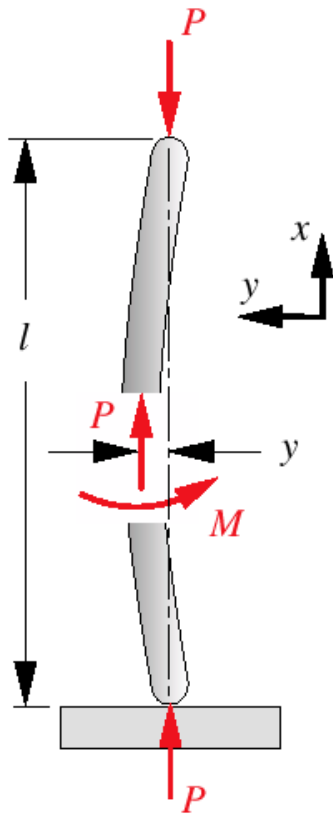
Installing a nacelle



Axial compression -- columns

Slenderness ratio: S_r

Buckling of a column



Definitions: $S_r = \frac{l}{k}$, with $k = \sqrt{\frac{I}{A}}$

- *Short columns:* $S_r < 10$

*Calculated based
on compression
stress criterion:*

$$\sigma_x = \frac{P}{A}$$

- *Intermediate/long
columns:* $S_r \geq 10$

*Calculated based
on critical unit load
criterion:*

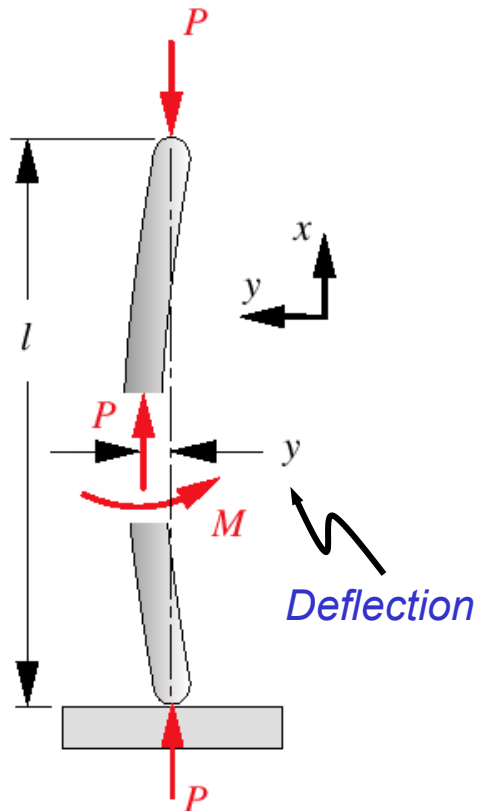
$$\frac{P_{cr}}{A}$$



Axial compression -- columns

Long columns: concentric load

Buckling of a column



Bending moment: $M = -P y$

For small deflections: $\frac{M}{EI} = \frac{d^2 y}{dx^2}$

(Governing ODE) $\implies \frac{d^2 y}{dx^2} + \frac{P y}{EI} = 0$

Solution (deflection) indicates:

$$y(x) = C_1 \sin\left(\sqrt{\frac{P}{EI}} x\right) + C_2 \cos\left(\sqrt{\frac{P}{EI}} x\right)$$



Axial compression -- columns

Long columns

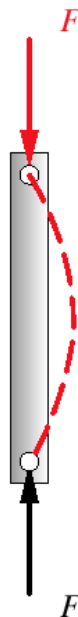
Deflection: $y(x) = C_1 \sin\left(\sqrt{\frac{P}{EI}} x\right) + C_2 \cos\left(\sqrt{\frac{P}{EI}} x\right)$

C_1 and C_2 are determined from
boundary conditions (end conditions)

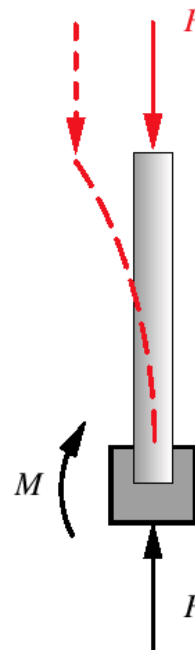
Possible end conditions (make sure you understand BC's in terms of slope and deflection):



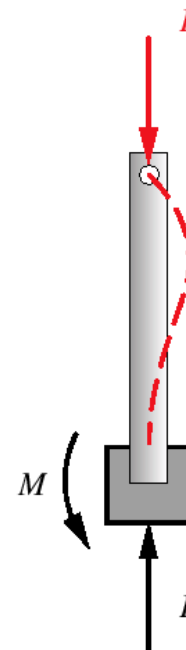
(a) Rounded-rounded



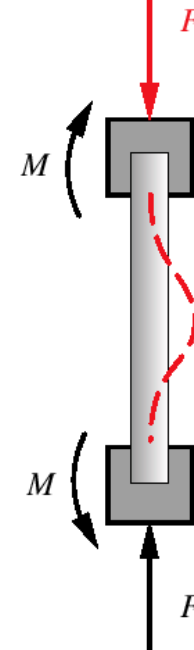
(b) Pinned-pinned



(c) Fixed-free



(d) Fixed-pinned



(e) Fixed-fixed



Axial compression -- columns

Long columns: end conditions + critical load P_{cr}

Deflection: $y(x) = C_1 \sin\left(\sqrt{\frac{P}{EI}} x\right) + C_2 \cos\left(\sqrt{\frac{P}{EI}} x\right)$

For the rounded-rounded end conditions:

(1) $y(x=0) = 0 \implies C_2 = 0$

(2) $y(x=l) = 0 \implies C_1 \sin\left(\sqrt{\frac{P}{EI}} l\right) = 0$

Indicating that:

$$\sqrt{\frac{P}{EI}} l = n \cdot \pi; \quad n = 1, 2, 3, \dots$$

Many solutions....

Therefore,

$$P_n = \frac{(n \cdot \pi)^2 E I}{l^2}; \quad n = 1, 2, 3, \dots$$

Many critical loads....



Axial compression -- columns

Long columns: end conditions + critical load P_{cr}

Typically, designs are based on the smallest critical load. Therefore,

$$P_{cr} = \frac{\pi^2 E I}{l^2}; \quad \text{for } n = 1$$

using: $I = A k^2$ and $S_r = \frac{l}{k}$



$$\frac{P_{cr}}{A} = \frac{\pi^2 E}{S_r^2}; \quad \text{for } n = 1$$

Critical load per unit area in terms of slenderness ratio

Corresponding deflection: $y = C_1 \sin\left(\frac{\pi x}{l}\right)$

For BC's:

(1) $y(x = 0) = 0$

(2) $y(x = l) = 0$



Axial compression -- columns

Accounting for different BC's

In order to take into account other boundary conditions, the concept of **effective length**, l_{eff} , is introduced:

$$S_r = \frac{l_{eff}}{k} \implies \boxed{\frac{P_{cr}}{A} = \frac{\pi^2 E}{S_r^2}}$$

Table 4-3 Column End-Condition Effective Length Factors

End Conditions	Theoretical Value	AISC* Recommended	Conservative Value
Rounded-Rounded	$l_{eff} = l$	$l_{eff} = l$	$l_{eff} = l$
Pinned-Pinned	$l_{eff} = l$	$l_{eff} = l$	$l_{eff} = l$
Fixed-Free	$l_{eff} = 2l$	$l_{eff} = 2.1l$	$l_{eff} = 2.4l$
Fixed-Pinned	$l_{eff} = 0.707l$	$l_{eff} = 0.80l$	$l_{eff} = l$
Fixed-Fixed	$l_{eff} = 0.5l$	$l_{eff} = 0.65l$	$l_{eff} = l$



Designing columns

General procedure

1) Determine force to be supported and expected length of the column (*design objective and corresponding constraint(s)*). Use free-body diagram(s) and equilibrium conditions.

2) Determine cross-section parameters of a proposed column:

- Area, A
- Moment of inertia, I
- Radius of gyration, k

3) Determine slenderness ratio, $S_r = \frac{l_{eff}}{k}$

- Identify boundary conditions (BC's) and apply appropriate value for the effective length l_{eff} -- use appropriate table (Table 4-3.)

4) Identify material to use and its corresponding compressive yield strength, S_{yc} , and elastic modulus, E .



Designing columns

General procedure

4) Determine slenderness ratio at half-yield: $(S_r)_D$ (**Design criterion**)

- $(S_r)_D$ is obtained as follows:

(a) Set the load per unit area as: $\frac{P_{cr}}{A} = \frac{S_{yc}}{2}$ (Half the yield strength value in compression)

(b) Therefore, $\frac{S_{yc}}{2} = \frac{\pi^2 E}{S_r^2}$

(c) Solve for slenderness ratio -- using previous equation, (b).

This is the slenderness ratio at half-yield in compression.

$$(S_r)_D = \pi \sqrt{\frac{2E}{S_{yc}}}$$



Designing columns

General procedure

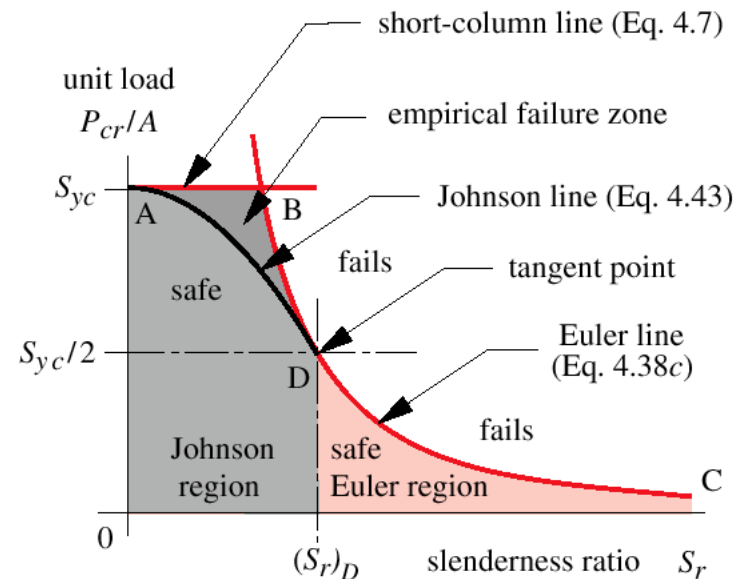
5) Determine type of column based on proposed design:

- Johnson: if S_r (step 3) < $(S_r)_D$ (Step 4)
- Euler: if S_r (step 3) > $(S_r)_D$ (Step 4)

6) Determine critical load

- Johnson:
$$P_{cr} = A \left[S_{yc} - \frac{1}{E} \left(\frac{S_{yc} S_r}{2\pi} \right)^2 \right]$$

- Euler:
$$P_{cr} = A \frac{\pi^2 E}{S_r^2}$$



(a) Construction of column failure lines



Designing columns

General procedure

7) Determine allowed load: $P_{allowed} = \frac{P_{cr}}{SF}$

- P_{cr} , is the critical load (step 6)
- SF , is the security factor (> 1)

8) If $P_{allowed}$ is > than force to be supported, then, a satisfactory design has been obtained -- *not necessarily the optimal* !! Go to step (1) and refine your design, if possible (e.g., weight minimization)

9) If $P_{allowed}$ is < than force to be supported, then, go to step (1) and improve design. You can select a different section and/or material.

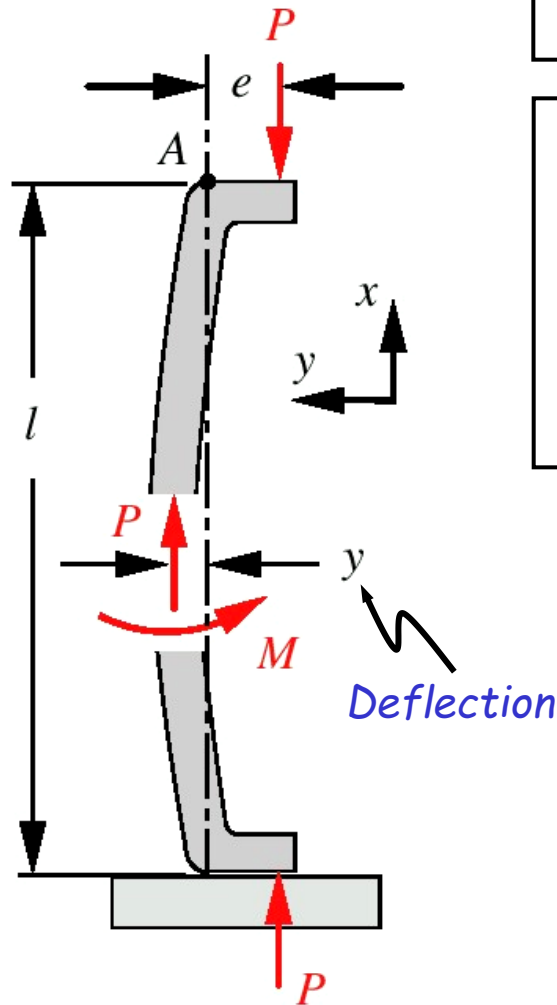
Use of MathCad is strongly recommended !!



Axial compression -- columns

Eccentrically loaded

Eccentrically loaded column



Moments: $\sum M_A = -M + P(e + y) = 0$

For small deflections: $\frac{M}{EI} = \frac{d^2 y}{dx^2}$

(Governing ODE) $\implies \frac{d^2 y}{dx^2} - \frac{P y}{EI} = \frac{P e}{EI}$

For BC's: $y(x = 0) = 0; \quad \left. \frac{dy}{dx} \right|_{x=\frac{l}{2}} = 0$

$\implies y\left(x = \frac{l}{2}\right) = e \left[\sec\left(\frac{l}{2} \sqrt{\frac{P}{EI}}\right) - 1 \right]$



Axial compression -- columns

Eccentrically loaded

Maximum moment: $M_{\max} = P \cdot \left(e + y \Big|_{x=\frac{l}{2}} \right) = P \cdot e \cdot \sec \left(\frac{l}{2} \sqrt{\frac{P}{EI}} \right)$

Compressive stress: $\sigma_c = \frac{P}{A} + \frac{M c}{I} = \frac{P}{A} + \frac{M c}{A k^2}$

with the maximum stress level at $M = M_{\max}$ which yields

$$\sigma_{c,\max} = \frac{P}{A} \left[1 + \left(\frac{e c}{k^2} \right) \cdot \sec \left(\frac{l}{k} \sqrt{\frac{P}{4EA}} \right) \right] \quad \text{setting} \quad \sigma_{c,\max} = S_{yc}$$

$$\Rightarrow \frac{P}{A} = \frac{S_{yc}}{1 + \left(\frac{e c}{k^2} \right) \cdot \sec \left(\frac{l_{\text{eff}}}{k} \sqrt{\frac{P}{4EA}} \right)}$$

Solve for P to obtain
the critical load
(Secant column formula)



Reading assignment

- Chapters 4 of textbook: Sections 4.12 to 4.19
- Review notes and text: ES2501, ES2502

Homework assignment

- **Author's:** as indicated on Website of our course
- **Solve:** as indicated on Website of our course

